SYMBOLS LEGEND

SYMBOL	DESCRIPTION
-	GRID LINES
X / XX	SECTION OR DETAIL CUT
	SHEET NUMBER
X	ELEVATION CUT
XX	SHEET NUMBER
XXX'-XX"	ELEVATION CALLOUT
X	DRAWING REVISION NUMBER
	CURRENT REVISION CLOUD

SYMBOL	DESCRIPTION
[XX.Xk]	MECH UNIT (XX.Xk = MECH UNIT OPERATING WEIGHT IN KIPS INCLUDING INERTIA BASE)
	EXTENT OF CONCRETE PAD
X"	PAD THICKNESS
	CAST-IN-PLACE CONCRETE
SOGXX	SLAB TYPE
	SLAB THICKNESS
XXX'-XX"	T/SLAB
F1	**F**TG MARK (SF = **S**TRIP **F**TG)
XX'-X"	T/FTG EL
CXX	**C**OLUMN SIZE OR MARK
BP-X	**B**ASE **P**LATE TYPE
WC#	WOOD COLUMN MARK (#), SEE **6/2.36**
WB#	WOOD BEAM MARK (#), SEE **9/2.36**

SYMBOL	DESCRIPTION
	SERVICE LOAD PROVIDED FOR SPECIALTY DESIGNER
	STEP
	SLOPE
1	KEY NOTE
	SUBGRADE
	FORM SAVER
	WELDED-WIRE REINFORCEMENT
	ROUGHENED SURFACE, INTENTIONALLY ROUGHEN TO 1/4" AMPLITUDE, UNO

SYMBOL	DESCRIPTION
	EXISTING WIDE LINE
	EXISTING MEDIUM LINE
	EXISTING THIN LINE
	EXISTING HIDDEN LINE
(E) TEXT	EXISTING TEXT
X'-XX" *	DIMENSIONS TO EXISTING STRUCTURE, FIELD VERIFY (ASTERIC)
X'-XX"	DIMENSIONS TO NEW STRUCTURE
	EXISTING FRAMING (HALFTONE)
	NEW FRAMING

NOTES:
1. ITEMS NOT DESIGNED BY M/M WYOMING ARE SHOWN HALFTONED,
2. ITEMS INCLUDE:
   - EXISTING CONSTRUCTION
   - PERFORMANCE SPECIFIED ITEMS
   - CONSTRUCTION SHORING (TEMPORARY)
   - CONSTRUCTION SOIL SHORING/AMENDMENT OF SUBGRADES (TEMPORARY)
   - SOIL SHORING/AMENDMENT OF SUBGRADES (PERMENANT)

ABBREVIATIONS

/	Per
@	At
AB	Anchor Bolt
ACI	American Concrete Institute
ADDNL	Additional
AESS	Architecturally Exposed Structural Steel
AFF	Above Finished Floor
ALT	Alternate
ALUM	Aluminum
APA	American Plywood Association
APPROX	Approximate
ARCH	Architect or Architectural
B/ or BO	Bottom of
BAL	Balance
BD	Board
BF	Braced Frame
BG	Backgouge
BL	Brick Ledge
BLDG	Building
BLKG	Blocking
BM	Beam
BN	Boundary Nail
BOS	Bottom of Steel
BOT or B	Bottom
BRG	Bearing
BSMT	Basement
BTWN	Between
CC	Center to Center
CF	Cold Formed
CG	Center of Gravity
CIP	Cast-In-Place
CJ	Control Joint
CJP	Complete Joint Penetration
CL	Centerline
CLG	Ceiling
CLMS	Ceiling/ Light/ Mechanical/ Superimposed Load
CLR	Clear
CMU	Concrete Masonry Unit
COL	Column
CONC	Concrete
CONN	Connection
CONST	Construction
CONT	Continue or Continuous
CONTR	Contractor
COORD	Coordinate
CSJ	Construction Joint
CTR(D)	Center(ed)
d	Penny
D or DL	Dead Load
DAS	Deformed Anchor Stud
DBL	Double
DCW	Demand Critical Weld
DFS	Deferred Submittal
Di	Gravity Ice Load
DIA or Ø	Diameter
DIAG	Diagonal
DIM	Dimension
DN	Down
DO	Ditto
DP	Drilled Pier or Deep
DT	Precast Double Tee
DTL(s)	Detail(s)
DWG(s)	Drawing(s)
DWL(s)	Dowels(s)

(E) or EXIST	Existing
E	Earthquake Load
E-W	East-West
EA	Each
EC	Epoxy Coated
EE	Each End
EF	Each Face
EJ	Expansion Joint
EL	Elevation
ELEV	Elevator
EMBED	Embedded
EN	Edge Nail
ENGR	Engineer
EOR	Engineer-of-Record
EQ	Equal
EQ SP	Equally Spaced
EQUIP	Equipment
ES	Each Side
EW	Each Way
EXP	Expansion
EXP ANCH	Expansion Anchor
EXT	Exterior
F	Fluid Load
Fa	Flood Load
FAB	Fabricate
FD	Footing Dowel
FF	Finished Floor
FIN	Finish(ed)
FLG	Flange
FLR	Floor
FND	Foundation
FO	Face of
FP	Full Penetration or Fire Proofing
FRAM	Framing
FS	Far Side
FT	Foot or Feet
FTG	Footing
FV	Field Verify
GA	Gage or Gauge
GALV	Galvanized
GC	General Contractor
GL	Glu-lam
GR	Grade or Grind
GR BM	Grade Beam
H	Soil Lateral Load
HAS or HDAS	Headed Anchor Stud
HD	Headed or Holdown
HDAR	Headed Anchor Rod
HDG	Hot Dipped Galvanized
HK	Hook
HORIZ	Horizontal
HT	Height
HVAC	Heating-Ventilating and A/C
I.F.	Inside Face
ID	Inside Diameter
IN	Inch
INT	Interior
IT	Precast Inverted Tee Beam
JST	Joist
JT	Joint
k	Kip

L	Length or Live Load
LB	Precast L-Shaped Beam
LB(S)	Pound(s)
LCE	Compression Embedment
LCS	Compression Lap Splice
LDH	Hook Development Length
LG	Length
LL	Live Load
LLH	Long Leg Horizontal
LLV	Long Leg Vertical
LOC(s)	Location(s) or Locate
LONG	Longitudinal
Lr	Roof Live Load
LSL	Laminated Strand Lumber
LT	Light
LTE	Tension Embedment
LTS	Tension Lap Splice Length
LTWT	Lightweight
LVL	Level or Laminated Veneer Lumber
LWC	Light Weight Concrete
MACH	Machine
MACH RM	Machine Room
MAS	Masonry
MATL	Material
MAX	Maximum
MBS	Metal Building Supplier
MCJ	Masonry Control Joint
MECH	Mechanical
MEP	Mech/Elect/Plumb
MIN	Minimum
MISC	Miscellaneous
MIL	Micro-Lam
MLS	Masonry Lap Splice
mm	Millimeter
MNFR	Manufacturer
MO	Masonry Opening
MTL	Metal
N	North
N-S	North-South
NIC	Not in Contract
NM	Non-Metallic
NO or #	Number
NOM	Nominal
NS	Non-Shrink or Near Side
NTS	Not To Scale
NWC	Normal Weight Concrete
O.F.	Outside Face
OAE	Or Approved Equivalent
OC	On Center
OD	Outside Diameter
OH	Opposite Hand
OPNG	Opening
OPP	Opposite
OVS	Oversized
OWS	One-Way Slab
PAF	Power Actuated Fastener
PC	Precast
PCA	Portland Cement Association
PD	Pier Dowel
PEN	Penetration
PERP	Perpendicular
PL	Plate (Steel)
PLF	Pounds Per Lineal Foot
PREFAB	Prefabricated
PRELIM	Preliminary
PS	Prestressed
PSF	Pounds Per Square Foot
PSI	Pounds Per Square Inch
PT	Point or Post-Tension or Pretensioned

QTY	Quantity
R	Radius or Rain Load
RAD	Radius
RB	Precast Rectangular Beam
RC	Reinforced Concrete
RE: or REF	Refer to (Reference)
REINF	Reinforce(ing)(d)(ment)
REQD	Required
REQT(s)	Requirement(s)
RET	Return
RO	Rough Opening
ROF	Random Oriented Fiber
(S)	Salvaged
S	South
SC	Slip Critical
SCHED	Schedule
SECT	Section
SIM	Similar
SL	Snow Load
SLH	Short Leg Horizontal
SLRS	Seismic Load Resisting System
SLV	Short Leg Vertical
SOG	Slab on Grade
SP	Space(s)
SP @	Space at
SPECS	Specifications
SPRT	Support
SS	Stainless Steel
STD	Standard
STIFF	Stiffener
STL	Steel
STR	Structural
SW	Shearwall
SYM	Symmetrical
T	Top or Thermal Load
T&B	Top and Bottom
T/ or T.O	Top of
THK	Thick or Thickness
TL	Total Load
TOC	Top of Concrete
TOF	Top of Footing
TOM	Top of Masonry
TOP	Topping
TOS	Top of Steel
TOW	Top of Wall
TRANS	Transverse
TWS	Two-Way Slab
TYP	Typical
ULT	Ultimate
UNO	Unless Noted Otherwise
Vasd	Service Level/ Nominal Design Wind Speed
VERT	Vertical
VIF	Verify in Field
Vult	Ultimate Design Wind Speed
W	Wind Load
W/	With
W/O	Without
WD	Width or Wood
WF	Wide Flange
Wi	Wind-on-Ice Load
WP	Working Point or Waterproofing
WT	Weight
WWR	Welded Wire Reinforcing
WxH	Width x Height

FOUNDATION NOTES

1) DESIGN CRITERIA:
1A) THE SOIL PARAMETERS USED IN DESIGN ARE FROM IBC 2015 TABLES 1806.2, WHICH ARE SPECIFIED BELOW.

2) FOOTINGS:
2A) CONTRACTOR TO ENGAGE GEOTECHNICAL ENGINEER TO VERIFY SOIL PARAMETERS USED IN DESIGN. SEE ARCHITECTURAL SPECIFICATIONS.

2B) FOOTING DESIGN CRITERIA:
- MAXIMUM TOTAL LOAD BEARING PRESSURE = 2000 PSF
- ULTIMATE COEFFICIENT OF FRICTION TO RESIST LATERAL LOADS = 0.30
- FROST DEPTH TO BOTTOM OF FOUNDATION = 42 IN

3) FOUNDATION WALLS:
3A) EQUIVALENT FLUID PRESSURES USED FOR WALL DESIGN:
- "ACTIVE" CONDITION = 45 PCF
- "AT REST" CONDITION = 60 PCF
- "PASSIVE" CONDITION = 200 PCF
- LATERAL PRESSURE DUE TO SURCHARGE = 5100 PLF AT NEW SUB-BASEMENT FND WALL
- ULTIMATE COEFFICIENT OF FRICTION TO RESIST LATERAL LOADS = 0.30

3B) BACKFILL MATERIAL ADJACENT TO FOUNDATION WALLS SHALL NOT IMPOSE GREATER LOADS THAN THOSE SHOWN ABOVE, WHICH WERE USED IN THE FOUNDATION WALL DESIGNS.

4) EXCAVATION:
4A) GEOTECHNICAL ENGINEER TO PROVIDE SOIL AMMENDMENTS AND/OR SHORING RECOMMENDATIONS AS REQUIRED TO STABALIZE SOILS UNDER AND ADJACENT TO EXISTING FOUNDATION ELEMENTS IN TEMPORARY AND PERMANENT CONDITIONS.

4B) DO NOT UNDERMINE EXISTING FOUNDATIONS.

POST-INSTALLED ANCHOR NOTES

1) PERSONNEL REQUIREMENTS:
1A) THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. SUBMIT DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS HAVE PASSED THE TRAINING COURSE PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.

1B) PERSONNEL WHO WILL INSTALL HORIZONTAL OR UPWARDLY INCLINED ADHESIVE ANCHORS IN CONCRETE THAT SUPPORT SUSTAINED TENSION LOADS SHALL BE CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. THESE ANCHORS ARE DESIGNATED WITH A (CERT) AFTER THE ANCHOR CALL OUT. SUBMIT DOCUMENTED CONFIRMATION THAT PERSONNEL HAVE PASSED THE TRAINING COURSE PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.

2) INSTALLATION REQUIREMENTS:
2A) ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED ACCORDING TO MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND PER MANUFACTURER'S ON-SITE TRAINING.

2B) ALL ADHESIVE ANCHORS AND ADHESIVE ANCHORED REINFORCEMENT DESIGNS ARE FOR INSTALLATION IN THE FOLLOWING CONDITIONS, UNLESS NOTED OTHERWISE. WRITTEN APPROVAL MUST BE RECEIVED FROM ENGINEER PRIOR TO INSTALLATION IN ALTERNATE CONDITIONS.
- DRY CONCRETE, UNLESS NOTED OTHERWISE.
- CONCRETE TEMPERATURE AT TIME OF INSTALLATION THROUGH CURE TIME MUST BE WITHIN THE TEMPERATURE RANGE SPECIFIED IN MANUFACTURER'S PRINTED INSTALLATION INSTRUCTION FOR ADHESIVE GEL AND CURE TIMES.
- ANCHOR HOLES TO BE HAMMER DRILLED AND CLEANED.
- CONCRETE MUST BE AT LEAST 21 DAYS OLD BEFORE INSTALLATION OF ANCHORS.
- HOLES TO BE CLEANED AND PREPARED IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND EVALUATION REPORT PRIOR TO ADHESIVE INJECTION.

2C) THE POSITION OF EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE SHALL BE LOCATED PRIOR TO INSTALLING POST INSTALLED ANCHORS OR REINFORCEMENT. EXISTING REINFORCEMENT SHALL BE LOCATED USING A SCANNER, GPR, X-RAY, CHIPPING OR OTHER MEANS. DO NOT DAMAGE OR CUT EXISTING REINFORCEMENT.

3) SUBSTITUTION REQUESTS:
3A) SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS AND PRODUCT DATA DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS IN COMPLIANCE WITH THE RELEVANT BUILDING CODES, LOAD RESISTANCE, INSTALLATION CATEGORY, CREEP APPROVAL, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE OF THE SPECIFIED PRODUCT.

POST-INSTALLED ANCHOR TABLE

ANCHOR TYPE	PRODUCT	Fy (KSI)	Fu (KSI)	COMMENT
ADHESIVE (IN CONCRETE)	HILTI HIT-HY 200	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS
ADHESIVE (IN CONCRETE W/>12" EMBEDMENT)	HILTI HIT-RE 500 V3	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS
ADHESIVE (IN GROUTED OR HOLLOW MASONRY)	SIMPSON SET HILTI HIT-HY 70	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS
ADHESIVE ANCHOR RODS	-	36 MIN	58 MIN	THREADED ROD, UNGREASED
EXPANSION ANCHORS (IN CONCRETE)	SIMPSON STRONG BOLT HILTI KWIK BOLT TZ	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS
EXPANSION ANCHORS (IN GROUTED MASONRY)	SIMPSON WEDGE-ALL HILTI KWIK BOLT 3	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS
SCREW ANCHORS	SIMPSON TITEN HD HILTI KWIK HUS-EZ	-	-	SUBMIT CALCULATIONS FOR SUBSTITUTIONS

DEFERRED SUBMITTALS

1) GENERAL:
1A) THE FOLLOWING PORTIONS OF THE STRUCTURAL DESIGN WILL NOT BE SUBMITTED AT THE TIME OF PERMIT APPLICATION. WHEN RECEIVED AND REVIEWED, THESE DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL BY THE CONTRACTOR:
- EXCAVATION SHORING
- HELICAL PIERS
- PILES
- METAL RAILINGS
- ANCHORAGE, BRACING AND ATTACHMENT OF REQUIRED ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, FIRE SPRINKLER, AND OTHER EQUIPMENT AND SYSTEMS.
- PERMANENT EARTH SHORING SYSTEMS

1B) LOADING AND LOCATION FOR ATTACHMENT OF DEFERRED SUBMITTAL ITEMS ARE NOTED ON DRAWINGS AND ARE NOT TO BE RE-LOCATED OR INCREASED WITHOUT WRITTEN APPROVAL.

1C) SUBMIT STAMPED STRUCTURAL CALCULATIONS FOR ALL DEFERRED SUBMITTAL ITEMS PRIOR TO OR CONCURRENTLY WITH DRAWINGS OR PRODUCT DATA. INCLUDE ANALYSIS OF ATTACHMENT TO PRIMARY STRUCTURE. INCLUDE CURRENT ICC REPORT WITH ALL PROPRIETARY STRUCTURAL ELEMENTS AND ANCHORS/FASTENERS.

1D) POWDER ACTUATED FASTENERS (PAF) SHALL NOT BE USED TO RESIST TENSION LOADS. POWDER ACTUATED FASTENERS SHALL NOT BE USED TO RESIST GRAVITY LOADS WHICH INCLUDE BRICK VENEER.

STRUCTURAL DRAWING LIST

SHEET NUMBER	SHEET TITLE
2.01	NOTES
2.02	NOTES
2.03	QUALITY ASSURANCE
2.10	SUB-BASEMENT
2.11	BASEMENT
2.12	FIRST FLOOR
2.13	SECOND FLOOR
2.14	ROOF FRAMING & ATTIC FRAMING
2.30	CONCRETE DETAILS
2.31	SUB-BASEMENT DETAILS
2.32	BASEMENT & UPPER FLOOR DETAILS
2.33	ELEVATOR PIT DETAILS
2.34	ELEVATOR & RETRO-FIT LINTEL DETAILS
2.35	STAIR DETAILS
2.36	WOOD & ROOF DETAILS

DESIGN CRITERIA

1) CODES AND STANDARDS:
1A) GENERAL DESIGN
- INTERNATIONAL BUILDING CODE 2015

1B) LOADS
- ASCE/SEI 7-10 "MINIMUM DESIGN LOAD FOR BUILDINGS AND OTHER STRUCTURES" WHERE INDICATED ON DRAWINGS INDIVIDUAL UNFACTORED LOAD COMPONENTS (D, Di, L, Lr, R, S, H, F, Fa, E, W, Wi, T) ARE AS DEFINED AND DETERMINED BY THE BUILDING CODES AND STANDARDS INDICATED. LOAD COMPONENTS SHALL BE COMBINED USING THE LOAD COMBINATIONS OF THE BUILDING CODE FOR SPECIALTY DESIGN BY OTHERS.

1C) CONCRETE
- ACI 301-10 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS"
- ACI 318-11 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"

1D) STEEL
- ANSI/AISC 341-10 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS"
- ANSI/AISC 360-10 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" LOAD AND RESISTANCE FACTOR DESIGN

2) SEISMIC LOADS
- SEISMIC DESIGN CATEGORY = B
- RISK CATEGORY = II
- EARTHQUAKE IMPORTANCE FACTOR, Ie = 1.00
- MAPPED SPECTRAL RESPONSE ACCELERATION, Ss = 17.80 %g
- MAPPED SPECTRAL RESPONSE ACCELERATION, S1 = 7.40 %g
- DESIGN SPECTRAL RESPONSE COEFFICIENT, SDs = 0.190
- DESIGN SPECTRAL RESPONSE COEFFICIENT, SD1 = 0.118
- SOIL SITE CLASS = D

3) WIND LOADS
- RISK CATEGORY = II
- BASIC ULTIMATE WIND SPEED, Vult = 120 mph
- BASIC NOMINAL WIND SPEED, Vasd = 93 mph
- EXPOSURE CATEGORY = B
- INTERNAL PRESSURE COEFFICIENT, Gcpi = +/-0.18
- TOPOGRAPHIC FACTOR, Kzt = 1.0

4) DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING AND ELEMENTS DESIGNED BY THE CONTRACTOR
4A) PRESSURES LISTED BELOW ARE ULTIMATE
4B) FINAL CALCULATIONS TO BE COMPLETED BY CONTRACTOR
4C) SEE 'WALL CORNER AND SPECIAL ROOF ZONES DIAGRAM'
4D) COMPONENT AND CLADDING SURFACE PRESSURES (PSF)

- **WALLS PRESSURES**

WALLS AREA	**10 SF**	**100 SF**	**500 SF**
WALLS INTERIOR NEG (ZONE 4)	-43.1	-37.2	-33.0
WALLS CORNER NEG (ZONE 5)	-53.2	-41.3	-33.0
WALLS POSITIVE ZONE 4 & 5	39.7	33.8	29.6

- **ROOF PRESSURES**

ROOF AREA	**10 SF**	**50 SF**	**100 SF**
ROOF INTERIOR NEG (ZONE 1) - NET UPLIFT FOR JOIST DESIGN UNO	-39.7	-35.0	-33.0
ROOF NEGATIVE (ZONE 2) - EAVES, RAKES, RIDGES	-46.5	-41.8	-39.7
ROOF CORNERS NEG (ZONE 3)	-46.5	-41.8	-39.7
ROOF POSITIVE ALL ZONES	36.4	34.0	33.0

5) GRAVITY LOADS
5A) SEE GRAVITY LOADS TABLE SEE LOAD KEY SHEETS FOR SUPERIMPOSED DEAD LOAD AND LIVE LOADS USED IN DESIGN

5B) DRIFTING, SLIDING AND UNBALANCED SNOW
- GROUND SNOW LOAD = 43.0 psf
- SNOW EXPOSURE FACTOR, Ce = 1.0
- SNOW LOAD IMPORTANCE FACTOR, Is = 1.0
- THERMAL FACTOR, Ct = 1.00

GRAVITY LOADS

LOCATION	SUPERIMPOSED DEAD LOAD (PSF)	LIVE LOAD (PSF)	LIVE LOAD REDUCTION	PARTITION LOAD (PSF)	POINT LOAD (LB)
ROOF	15	20 MIN UNIFORM LOAD, SEE NOTE 5 FOR SNOW LOADS	NO	--	300
GALLERIES/DISPLAY AREAS	10	100	YES	--	2000
ASSEMBLY AREAS, AUTIDTORIUM, LOBBIES	10	100	NO	--	2000
OFFICE	10	50	YES	15	2000
STAGE	10	150	YES	--	2000
TABLE & STAIR STORAGE	10	150	YES	--	--
GIFT SHOP	10	100	YES	--	2000
MECHANICAL	10	75 + EQUIP BUT NOT LESS THAN 150	NO	--	--

GENERAL NOTES

1) GENERAL:
1A) ENGINEER: REFERENCES ON THE STRUCTURAL DRAWINGS TO 'ENGINEER' MEAN THE STRUCTURAL ENGINEER OF RECORD. OTHER ENTITIES ARE SPECIFICALLY NOTED AS "CONTRACTOR'S ENGINEER", "MECHANICAL ENGINEER", ETC.

1B) THESE NOTES SUPPLEMENT THE SPECIFICATIONS, WHICH SHALL BE REFERENCED FOR ADDITIONAL REQUIREMENTS.

1C) UNDERGROUND UTILITIES: LOCATE EXISTING UTILITIES AND NOTIFY ARCHITECT OF EXISTING UTILITIES OR SUBGRADE CONDITIONS WHICH INTERFERE WITH WORK.

1D) STRUCTURAL ELEMENTS ARE CENTERED ON GRID LINES AND GRID LINE INTERSECTIONS UNLESS DIMENSIONED OTHERWISE.

2) EXISTING STRUCTURES:
2A) CONTRACT DOCUMENTS HAVE BEEN PREPARED USING AVAILABLE DRAWINGS AND SITE OBSERVATION AS PERMITTED BY ACCESS RESTRICTIONS DURING DESIGN.

2B) DURING CONSTRUCTION, THE CONTRACTOR MAY ENCOUNTER EXISTING CONDITIONS WHICH ARE NOT NOW KNOWN OR ARE AT VARIANCE WITH PROJECT DOCUMENTATION. CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ALL CONDITIONS NOT PER THE CONTRACT DOCUMENTS. EXAMPLES INCLUDE:
- SIZES OR DIMENSIONS OTHER THAN THOSE SHOWN
- DAMAGE OR DETERIORATION TO MATERIALS AND COMPONENTS
- CONDITIONS OF INSTABILITY OR LACK OF SUPPORT
- ITEMS NOTED AS EXISTING ON THE DRAWINGS BUT NOT FOUND IN THE FIELD

2C) PREPARE DIMENSIONAL DRAWINGS OF ALL DISCOVERED ITEMS.

2D) CONTRACTOR SHALL FIELD VERIFY ALL EXISTING STRUCTURAL CONDITIONS PRIOR TO SUBMITTING SHOP DRAWINGS.

2E) CONTRACTOR SHALL MAKE ALLOWANCE FOR THE RESOLUTION OF SUCH DISCOVERIES IN THE CONSTRUCTION SCHEDULE.

2F) SUBMIT A DIMENSIONED DRAWING OF ALL NEW OPENINGS THROUGH EXISTING STRUCTURE AND SECURE APPROVAL PRIOR TO CUTTING. DRAWING SHALL SHOW VERTICAL & HORIZONTAL LOCATION AND SIZE OF PROPOSED OPENING.

3) USE OF DRAWINGS:
3A) DO NOT SCALE DRAWINGS.

3B) WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED TYPICAL APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

4) TEMPORARY CONDITIONS:
4A) THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES. REFER TO "LATERAL LOAD RESISTING SYSTEM DESCRIPTION" IN DESIGN CRITERIA FOR ADDITIONAL INFORMATION.

4B) CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

4C) FOUNDATIONS AND STRUCTURAL ELEMENTS ADJACENT TO NEW WORK SHALL BE STABILIZED BY CONTRACTOR PRIOR TO THE DISTURBING EXISTING SOILS OR AS RECOMMENDED BY THE GEOTECHINAL ENGINEER. HAND TOOLS AND LIMITED VIBRATION ADJACENT TO EXISTING STRUCTURES IS RECOMMENDED.

4D) TEMPORARY SHORING LOADS ARE BASED ON ESTIMATED SELF WEIGHTS OF 10 PSF, CONSTRUCTION LIVE LOADS OF 20 PSF, AND GROUND SNOW LOADS OF 30 PSF. IF THESE LOADS ARE EXCEEDED, SHORING SHALL BE DESIGNED TO ACCOMODATE ACCORDINGLY.

5) SUBMITTALS AND SUBSTITUTIONS:
5A) SUBMITTALS: REFER TO SPECIFICATIONS FOR DETAILED REQUIREMENTS.
- IF THE CONTRACTOR REQUESTS A CHANGE FROM THE STRUCTURAL DRAWINGS, IT SHALL BE APPROVED BY THE ARCHITECT AND DESIGNED BY MARTIN/MARTIN WYOMING, INC. PRIOR TO SUBMITTING SHOP DRAWINGS. VARIATION SHALL BE INDICATED ON THE SHOP DRAWINGS. CONTRACTOR SHALL COMPENSATE MARTIN/MARTIN WYOMING, INC. FOR MAKING THE CHANGE.
- CONSTRUCTION DOCUMENTS SHALL NOT BE REPRODUCED FOR USE IN SUBMITTALS
- ALL SHOP DRAWINGS SHALL REFERENCE THE STRUCTURAL DRAWING NUMBER AND DETAIL USED TO PREPARE THE SUBMITTAL
- SUBMIT A STATEMENT OF RESPONSIBILITY FOR CONSTRUCTION OF THE LATERAL LOAD RESISTING SYSTEM IDENTIFIED IN THE DESIGN CRITERIA IN ACCORDANCE WITH IBC 2015 SECTION 1704

5B) SUBSTITUTIONS: ARCHITECTS APPROVAL SHALL BE SECURED FOR ALL SUBSTITUTIONS

5C) NONCONFORMANCE: NOTIFY ARCHITECT OF CONDITIONS NOT CONSTRUCTED PER THE CONTRACT DOCUMENTS PRIOR TO PROCEEDING WITH CORRECTIVE WORK. SUBMIT PROPOSED REPAIR TO THE ARCHITECT FOR ACCEPTANCE. CONTRACTOR SHALL COMPENSATE MARTIN/MARTIN WYOMING, INC. FOR DESIGNING THE REPAIR.

5D) ALL SHOP DRAWINGS SHALL BE SUBMITTED IN 24x36, 11x17 AND 8-1/2x11 FORMAT ONLY.

5E) ALL SHOP DRAWINGS SHALL BE SUBMITTED IN ELECTRONIC FORMAT ONLY.

6) OSHA STANDARDS:
6A) THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. NOTHING SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CONSTRUED AS ELIMINATING THE NEED FOR THE CONTRACTOR TO COMPLY WITH ALL OSHA REQUIREMENTS.

6B) THE CONTRACTOR SHALL ADD ALL NECESSARY BOLTS, ANCHOR BOLTS, PLATES, STIFFENER PLATES, STABILIZER PLATES, BRIDGING, BRACING, BEARING SEATS, COLUMN SPLICES, ETC., AS WELL AS CLOSURES FOR OPENINGS. IN ADDITION, FIELD WELD ANYTHING THAT MAY BE CONSIDERED A TRIP HAZARD, SUCH AS SHEAR STUDS, AFTER PROTECTIVE DECKING IS INSTALLED.

6C) WASHERS OR RINGS MAY BE WELDED TO COLUMNS TO PROVIDE FOR SAFETY CABLES. HOLES IN COLUMNS FOR SAFETY CABLES SHALL BE SHOP INSTALLED, AND SHALL BE INDICATED ON SHOP DRAWINGS. ADJUST COLUMN SPLICE LOCATIONS OR ADD COLUMN SPLICES AS NECESSARY TO COMPLY WITH OSHA REQUIREMENTS. SUBMIT PROPOSED LOCATIONS.

6D) ALL METAL JOISTS REQUIRED BY OSHA TO BE BOLTED SHALL HAVE ERECTION BOLTS INSTALLED REGARDLESS OF FINAL CONNECTION SHOWN ON THE STRUCTURAL DRAWINGS.

6E) WHERE THE STRUCTURAL DRAWINGS APPEAR TO CONFLICT WITH OSHA REQUIREMENTS, THE STRUCTURAL DRAWINGS REPRESENT FINAL CONDITIONS ONLY. THE CONTRACTOR SHALL ADD ALL ERECTION FRAMING NECESSARY TO COMPLY WITH OSHA.

7) CONSTRUCTION ENGINEERING:
7A) THE STRUCTURE DEFINED ON THE CONTRACT DOCUMENTS HAS BEEN DESIGNED ONLY FOR LOADS ANTICIPATED ON THE STRUCTURE DURING ITS SERVICE LIFE. PROVIDE ALL REQUIRED ENGINEERING AND OTHER MEASURES TO ACHIEVE THE MEANS, METHODS, AND SEQUENCES OF WORK. SUCH ENGINEERING MAY INCLUDE, BUT IS NOT LIMITED TO:
- LAYOUT
- DESIGN FOR FORMWORK, SHORING, AND RESHORING
- DESIGN OF CONCRETE MIXES
- ERECTION PROCEDURES WHICH ADDRESS STABILITY OF THE FRAME DURING CONSTRUCTION
- WELD PROCEDURES
- DESIGN OF TEMPORARY BRACING OF WALLS FOR WIND, SEISMIC, OR SOIL LOADS
- SURVEYING TO VERIFY CONSTRUCTION TOLERANCES
- EVALUATION OF TEMPORARY CONSTRUCTION LOADS ON STRUCTURE DUE TO EQUIPMENT AND MATERIALS
- STRUCTURAL ENGINEERING TO RESIST ANY OTHER LOADS NOT IDENTIFIED ON DESIGN DRAWINGS

8) COORDINATION:
8A) STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO SHOP DRAWINGS AND WORK.

8B) COORDINATE DIMENSIONS OF ALL OPENINGS, BLOCKOUTS, DEPRESSIONS, ETC., WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL.

8C) SEE ARCHITECTURAL PLANS FOR INTERIOR PARTITIONS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE IN SUCH A WAY SO AS TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 OF THE FLOOR FRAMING. DO NOT MAKE RIGID VERTICAL AND HORIZONTAL CONNECTIONS TO THE PRIMARY STRUCTURE IN THE PLANE OF THE PARTITION.

Professional Engineer (Structural)
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PROJECT
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DATE
11/20/17

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